



Tuned Mass Dampers for Response Reduction of a Reinforced Concrete Chimney Under Near-Fault Pulse-Like Ground Motions

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The article investigates response mitigation of a reinforced concrete (RC) chimney subjected to pulse-like near-fault ground motions using tuned mass damper (TMD) schemes. The total height of the chimney is 265 m with a mass of 11,109 ton. Three TMD schemes are used: single tuned mass damper (STMD), multiple TMDs having equal stiffness (w-MTMDs) and multiple TMDs having equal masses (e-MTMDs). The STMD is tuned to the fundamental frequency of the chimney while both w-MTMDs and e-MTMDs have three TMDs for controlling each of the first and second modes (total of six TMDs) of vibration. Response of the uncontrolled and controlled structures is calculated for 69 recorded ground motions containing a dominant velocity pulse. Displacement and acceleration at top node of the RC chimney are the response of interest for performance assessment. It is found that e-MTMDs are more effective and robust than other schemes. It is also found that the pulse period of ground motion plays a very important role in how effective the control schemes are. There is a large variability in the reduction of response across these ground motions, and optimization methods independent of ground motion are not robust. There is a need for more advanced optimization methods incorporating information about local seismic sources.

Keywords: chimney, earthquake, reinforced concrete (RC), tuned mass dampers, vibration, near-fault ground motion

INTRODUCTION

Reinforced concrete (RC) chimneys are tall structures. They are very sensitive to earthquake ground motion Wilson (2003) explained that seismic response of a tall chimney is not adequately described by a single degree of freedom (SDOF) model, and that significant contribution from higher modes can be expected, especially in acceleration response. Distribution of shear force and moment demands along the height of such chimneys, is therefore, not as is implied by many design standards which typically rely on the mode shape of the fundamental normal mode.

Seismic vibration mitigation of tall structures has been an active area of research. Longarini and Zucca (2014) studied a 50 m tall masonry chimney and found that tuned mass damper (TMD) can be used to reduce its seismic response. Multiple TMDs (MTMDs) could be more beneficial to control a range of frequencies, which provides robustness

against detuning caused by variations of the frequencies of the structure (Wang and Shi, 2019). Elias et al. (2016) showed that multiple TMDs tuned to different vibration modes and distributed over the structure (d-MTMDs) are superior to a single TMD tuned to the fundamental mode of vibration in controlling seismic response of tall chimneys. Application of d-MTMDs for wind response control of a chimney is presented in Elias et al. (2017, 2019a). The concept of d-MTMDs is well-established and defined for response control of different structures subjected to different loading conditions (Gill et al., 2017; Elias, 2018, 2019; Elias and Matsagar, 2019; Elias et al., 2019b; Matin et al., 2019; Nigdeli and Bekdas, 2019). Etedali et al. (2019) demonstrated that friction TMDs (FTMDs) can mitigate structural response better than TMDs. Other solutions such as inerter TMDs or particle dampers (Greco and Marano, 2013; Reggio and Angelis, 2015; De Domenico and Ricciardi, 2018a,b; De Domenico et al., 2018; Lu et al., 2018; Rezaee and Aly, 2018; Cao and Li, 2019) have also been proposed in the vibration control literature. Different optimization techniques are used by researchers to improve the performance of TMDs (Nigdeli and Bekdas, 2013, 2017; Bekdas et al., 2018; Yucel et al., 2019). Huergo and Hernández (2019a,b) presented a structural model using coupled shear and flexural beams incorporating an arbitrary number of TMDs along the height. A detailed literature survey on passive TMDs is provided by Elias and Matsagar (2017).

Ground motions close to an earthquake source are often impulsive. Forward-rupture directivity results in constructive interference of seismic waves radiated by different sections of a rupturing fault as they arrive at a station, and this is manifest as a strong, short duration velocity pulse (see, for example, Rupakhety et al., 2011). Such dominant pulses have been found to be demanding on tall and flexible structures. For example, Sigurðsson et al. (2019) have shown that near-fault pulse-like ground motions are more demanding than far-fault non-pulse-like ground motion of same peak acceleration to a utility-scale land-based wind turbine. Since the strong shaking in these pulse-like ground motions is impulsive in nature, TMD masses do not have enough time to respond to the excitation and thereby control structural response effectively. This has been generally acknowledged in the literature (see for example Chen and Wu, 2001). Matta (2013), however, showed that TMDs can be used to control response of structures to pulse-like ground motion, provided the TMD has a large enough mass. The potential effectiveness of TMDs also depends on the structure being considered. For example, effectiveness of TMDs might be better for tall and flexible structures, which are very susceptible to pulse-like ground motions, than for very stiff structures. Effectiveness of TMDs in controlling response of tall structures like chimneys when subjected to pulse-like ground motions is missing in the literature. This work investigates this important issue using numerical simulation of response of a tall reinforced concrete chimney subjected to many near-fault pulse-like ground motions recorded during past earthquakes. To quantify the effect of such ground motions, a set of 69 recorded earthquake ground motions are selected from the database given in Sigurðsson et al. (2019), and further described in Rupakhety (2010).

Three TMD schemes namely single TMD (STMD); MTMDs with equal stiffness (w-MTMDs); and MTMDs with equal masses

(e-MTMDs) are considered in this study. The STMD is tuned to the fundamental frequency of the chimney, while w-MTMDs and e-MTMDs are designed to control the response around two modes.

MATHEMATICAL MODELING

A 265 m high reinforced concrete (RC) chimney which has been studied by several investigators and originally reported by Melbourne et al. (1983) is considered as the case-study structure. Inelastic deformations and soil-structure interaction are ignored in this study. Schematic representation of the chimney with and without TMDs is presented in **Figure 1**. Further details about the structure can be found in Melbourne et al. (1983). **Figure 1A** shows the basic geometry of the structure. A typical cross section of the chimney and a schematic of a typical tuned mass damper is shown in **Figure 1B**. The structure is modeled with 30 2-dimensional uniform Euler-Bernoulli beam elements. The geometrical details of these 30 sections are given in **Table 1**.

Each node has one translational and one rotational degree of freedom, which is statically condensed. The base of the chimney is assumed to be fixed. A lumped mass matrix formulation is used. The statically condensed stiffness matrix is denoted as $[K_N]$ and the corresponding diagonal mass matrix is denoted by $[M_N]$. The damping matrix ($[C_N]$) is constructed in such a way that damping ratio in all the modes of vibration is 5% of critical damping.

Three different TMD schemes are considered. In the STMD scheme, a single TMD is placed at the top of the chimney. In the MTMD scheme, multiple TMDs are placed at the top of the chimney. The mass, stiffness, and damping coefficient of the i^{th} TMD are denoted by m_i , k_i , and c_i , respectively. The structural matrices with the TMDs incorporated into them are denoted by $[K_s]$, $[C_s]$, and $[M_s]$, and are of order $(N+n) \times (N+n)$ where N is 30 and n represents the number of TMDs used.

The equations of motion for the coupled system, under ground motion can be stated as,

$$[M_s]\{\ddot{x}_s\} + [C_s]\{\dot{x}_s\} + [K_s]\{x_s\} = -[M_s]\{r\}\ddot{x}_g \quad (1)$$

where the displacement, velocity, and acceleration vectors are denoted by $\{x_s\}$, $\{\dot{x}_s\}$, and $\{\ddot{x}_s\}$; \ddot{x}_g is the ground acceleration and $\{r\}$ is the influence coefficient vector populated by 1. The system of equations is solved by the Newmark's integration method.

The optimum parameters (frequency tuning ratio and damping ratio) of the TMDs are based on a detailed parametric study explained in the following sections. Two different types of MTMDs are considered: e-MTMDs where all the TMDs have the same mass, and w-MTMDs where all the TMDs have the same stiffness.

The multiple TMDs are tuned to the first two modes of vibration, and their frequencies are uniformly distributed around the corresponding modal frequencies. The natural frequency of each TMD (ω_i) is expressed by (Li, 2000; Li and Qu, 2006; Han and Li, 2008),

$$\omega_i = \omega_T \left[1 + \left(i - \frac{n+1}{2} \right) \frac{\beta}{n-1} \right] \quad i = 1 \text{ to } n \quad (2)$$

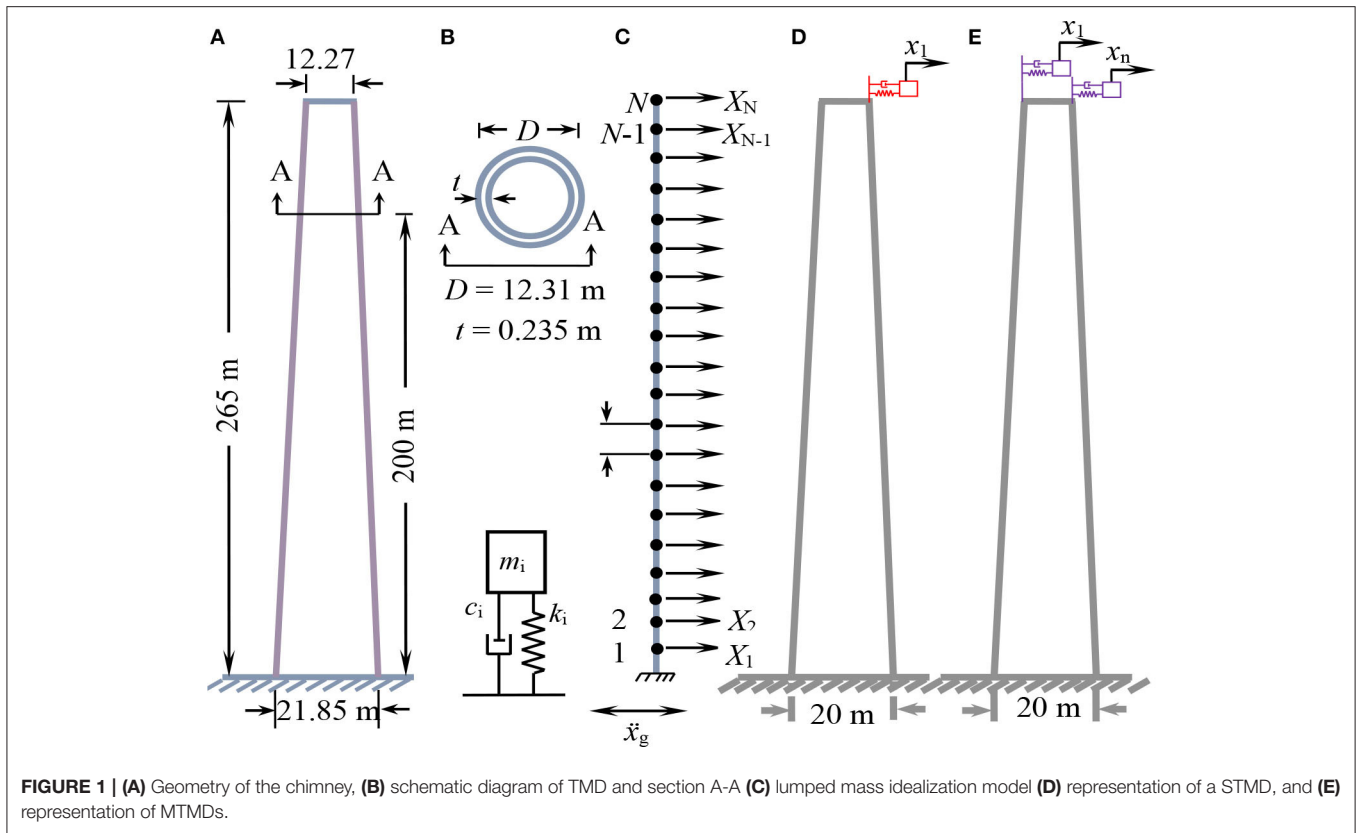


FIGURE 1 | (A) Geometry of the chimney, (B) schematic diagram of TMD and section A-A (C) lumped mass idealization model (D) representation of a STMD, and (E) representation of MTMDs.

and

$$\omega_T = \sum_i^n \frac{\omega_i}{n} \tag{3}$$

$$\beta = \frac{\omega_n - \omega_1}{\omega_T} \tag{4}$$

where ω_T is the average frequency of MTMDs and β is the non-dimensional frequency bandwidth of the MTMDs systems (for more details see Li, 2000). The average frequency of the MTMDs are tuned to the first two frequencies of the structure, and are denoted as ω_{T1} , ω_{T2} and n is equal to 6, implying that 3 TMDs are used to control each mode. For e-MTMDs, the mass of each TMD $m_i = \frac{m_t}{n}$, where m_t is the total mass of the TMDs. The stiffness (k_i) is then given by,

$$k_i = m_i \omega_i^2 \quad i = 1 \text{ to } n. \tag{5}$$

For w-TMDs, the stiffness of each TMD is given by

$$k_i = \frac{m_t}{\left(\frac{1}{\omega_1^2} + \frac{1}{\omega_2^2} + \dots + \frac{1}{\omega_n^2}\right)} \quad i = 1 \text{ to } 6 \tag{6}$$

$$m_i = \frac{k_i}{\omega_i^2} \tag{7}$$

The damping ratio ($\zeta_d = \zeta_1 = \zeta_2 = \dots = \zeta_n$) of the TMDs is kept the same and the damping coefficients (c_i) of the TMDs is

calculated as follows:

$$c_i = 2\zeta_d m_i \omega_i \quad i = 1 \text{ to } n \tag{8}$$

The total mass of the TMDs is assumed to be 3% of the total mass of the chimney, which is 11,109 ton. The resulting design parameters of the different TMD schemes are shown in **Table 2**. The first three frequencies of the uncontrolled chimney are 1.6798 rad/sec (0.267 Hz), 6.816 rad/sec (1.085 Hz) and 16.090 rad/sec (2.562 Hz). The corresponding modal mass participation for the three modes are 0.321, 0.193, and 0.112, respectively. Optimal frequency tuning ratio and damping ratio for STMD are 0.85 and 0.3, respectively. In case of w-MTMDs, the optimal frequency tuning ratio is found to be the same as STMD, but the optimal damping ratio is 0.18. Also, the frequency bandwidth of the three TMDs tuned to fundamental mode is 0.2 and that of those tuned to the second mode is 0.1. Frequency tuning ratio and damping ratio of the e-MTMDs 0.95 and 0.11, respectively. In this case, the bandwidth of all the TMDs is 0.3. These parameters were obtained by a detailed parametric study discussed in the next section.

NUMERICAL STUDY

Numerical simulation of response of the Chimney without and with different TMD schemes are carried using 69 near-fault pulse-like ground motions recorded during different past earthquakes. These ground motions were all affected by forward

TABLE 1 | Geometric details of the RC chimney.

Node number	Distance from base (m)	<i>l</i> (m)	Outer Diameter, <i>D</i> (m)	Thickness, <i>t</i> (m)	Mass, <i>m</i> (ton)
30	265	10	12.27	0.235	106.622
29	255	15	12.27	0.235	266.554
28	240	10	12.27	0.235	266.554
27	230	10	12.27	0.235	213.243
26	220	10	12.27	0.235	213.243
25	210	5	12.27	0.235	159.932
24	205	5	12.27	0.235	106.710
23	200	5	12.31	0.235	107.065
22	195	5	12.39	0.235	107.773
21	190	5	12.51	0.235	108.836
20	185	5	12.67	0.235	110.789
19	180	6	12.85	0.24	125.330
18	174	14	13.15	0.24	237.233
17	160	10	13.85	0.24	294.136
16	150	10	14.35	0.243	259.557
15	140	10	14.85	0.25	274.718
14	130	10	15.35	0.255	291.693
13	120	10	15.85	0.265	311.489
12	110	10	16.35	0.275	333.400
11	100	10	16.85	0.285	358.540
10	90	4	17.35	0.303	267.735
9	86	16	17.85	0.322	440.438
8	70	10	18.35	0.348	609.216
7	60	10	18.85	0.38	525.330
6	50	10	19.35	0.4	571.699
5	40	10	19.85	0.42	611.848
4	30	15	20.35	0.43	959.347
3	15	3	21.1	0.7	806.209
2	12	6	21.25	0.7	489.853
1	6	6	21.55	0.7	660.262
0	0	0	21.85	0.7	334.000

TABLE 2 | Design parameters of TMDs.

Schemes	TMDi	Frequency (rad/sec)	Mass (ton)	Stiffness 10 ⁴ (N/m)	Damping 10 ⁴ (N-sec/m)
STMD	TMD1	1.428	333.270	67.944	28.551
w-MTMDs	TMD1	1.285	126.531	20.895	5.854
	TMD2	1.428	102.490		5.268
	TMD3	1.571	84.703		4.789
	TMD4	4.925	8.615		0.764
	TMD5	5.794	6.224		0.649
	TMD6	6.663	4.707		0.564
e-MTMDs	TMD1	1.197	55.545	7.957	0.798
	TMD2	1.596		14.145	1.064
	TMD3	1.995		22.102	1.330
	TMD4	4.857		131.012	3.237
	TMD5	6.475		232.911	4.316
	TMD6	8.094		363.923	5.395

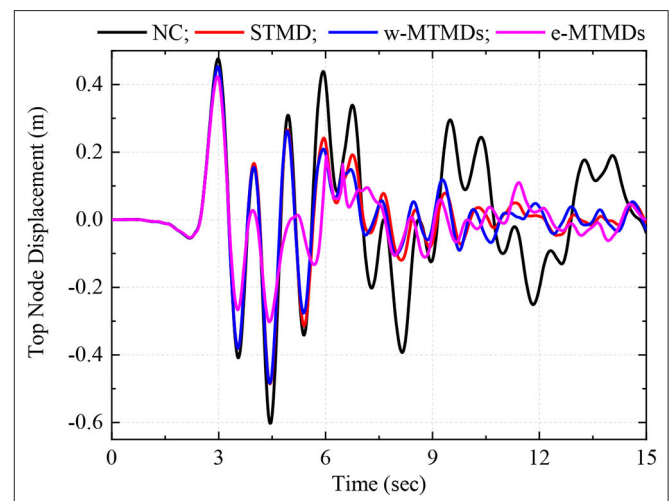


FIGURE 2 | Time history of top node displacement response of chimney with and without TMD schemes.

directivity effect and contain a dominant velocity pulse. Some salient features of the ground motions relevant for this study are given in **Table A1**.

Parameters of the TMDs were optimized through a parametric study. In the first step, tuning frequency was assumed to be equal to 1, and damping ratio of the TMD was varied from 0.01 to 0.5, with an increment of 0.01. The damping ratio corresponding to the best average response reduction across the 69 ground motions was selected as the optimal solution. Once the damping ratio is fixed, the frequency tuning ratio was varied between 0.85 and 1.15 with an increment of 0.01. The optimal solution was selected as the one that provided the best average response reduction across the 69 ground motions. Performance of TMDs

An example from one of the ground motions is presented here. The ground motion is from the 2004 Parkfield earthquake, 2004 earthquake recorded at the fault zone 12 station. The time variation of top node displacement of the uncontrolled (NC) and controlled structures is shown in **Figure 2**.

The peak top node displacement is 0.602, 0.486, 0.484, and 0.42 m respectively for NC, STMD, w-MTMDs, and e-MTMDs. The STMD, w-MTMDs and e-MTMDs reduce the top node displacement by 19, 19.6, and 30%, respectively. The time history of top node acceleration is presented in **Figure 3**. Response reduction in acceleration is 11.6, 10.5, and 49.6% respectively for STMD, w-MTMDs, and e-MTMDs.

The difference in the performance of the different schemes can be understood by inspecting the response in the frequency domain. Fourier amplitude spectra (FAS) of the top node displacement and top node acceleration are plotted in **Figures 4A, 5A**. The corresponding transfer functions are shown in **Figures 4B, 5B**. These transfer functions relate ground acceleration to top node displacement and acceleration, respectively. For ease of visualization, the transfer functions are normalized by the peak of the transfer function of the uncontrolled structure.

Figure 4 shows that the displacement response is mainly contributed by the first and the second modes of vibration. All the TMD schemes are equally effective in controlling the response due to the first mode. However, e-MTMDs outperform the other schemes in controlling the second mode. In this also evident from the transfer functions shown in **Figure 4B**. Because there is significant contribution from the second vibration mode, the e-MTMDs is more effective in controlling overall displacement response. Although the w-MTMDs are also intended to control the second mode, they are not as effective as the e-MTMDs because the TMD masses tuned to the second mode are rather low. Second mode contribution is even higher in acceleration response as shown in **Figure 5A**. The second mode contributes a lot more than the first in the acceleration response. The pulse

period of this ground motion is about 1 Hz which resonates with the second mode of the structure. Therefore, the e-MTMDs, which control the second mode the best, are the most effective in controlling the overall response. Apart from the pulse, high frequency part of the near-fault ground motion can also have significant impact on structure response (Rupakhety and Sigbjörnsson, 2011).

Next we discuss a scenario when the pulse period of the ground motion is close to the fundamental period of vibration of the chimney. Ground motion from the 1992 Landers earthquake recorded at the LUC station has a dominant pulse frequency of about 0.25 Hz. Top node displacement and acceleration due to this ground motion are shown in **Figure 6**. The TMDs are effective in controlling the displacement response, but not so effective in controlling acceleration. Unlike in the previous case where the e-MTMDs were found to be the best scheme, the w-MTMDs perform the best against this ground motion. This is because the w-MTMDs control the first vibration mode the best as a lot of mass is provided to those TMDs that are tuned to this mode. From the FAS of the response presented in **Figure 7**, it is clear that the displacement response is dominated by the first mode of vibration while the third mode has significant contribution in the acceleration response. These results show that the pulse period of near-fault ground motion relative to the vibration frequencies of the structure plays an important role in the structural response. This effect is further investigated in the next section.

Effect of Frequency Content of Ground Motions

To study the effect of pulse period response reduction ratios are plotted against the first mode period of the chimney normalized by the pulse period of 69 near-fault ground motions is shown in **Figure 8**. In controlling the peak displacement, the TMD schemes are, in general, not very effective, except when the ground motion pulse resonates with the structure. In a narrow

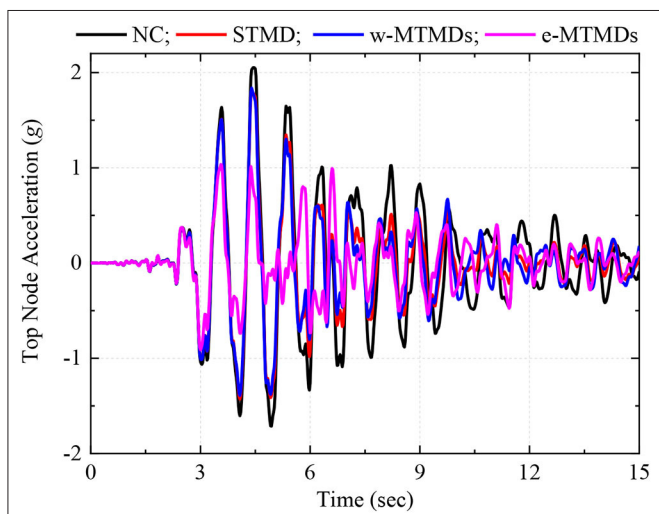


FIGURE 3 | Time history of top node acceleration response of chimney with and without TMD schemes.

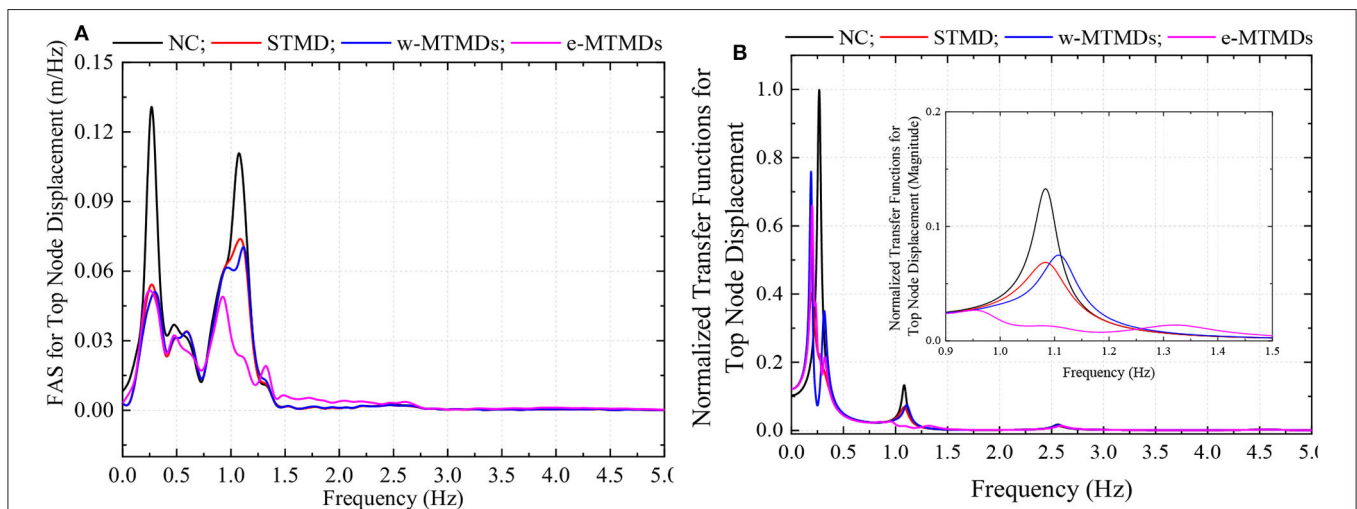
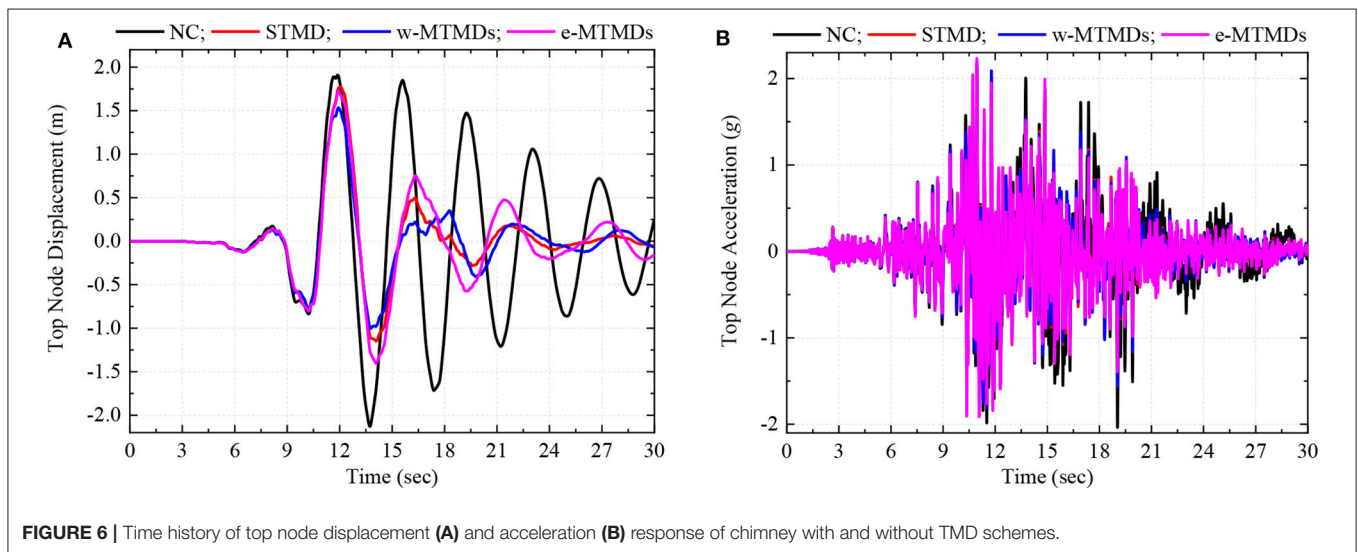
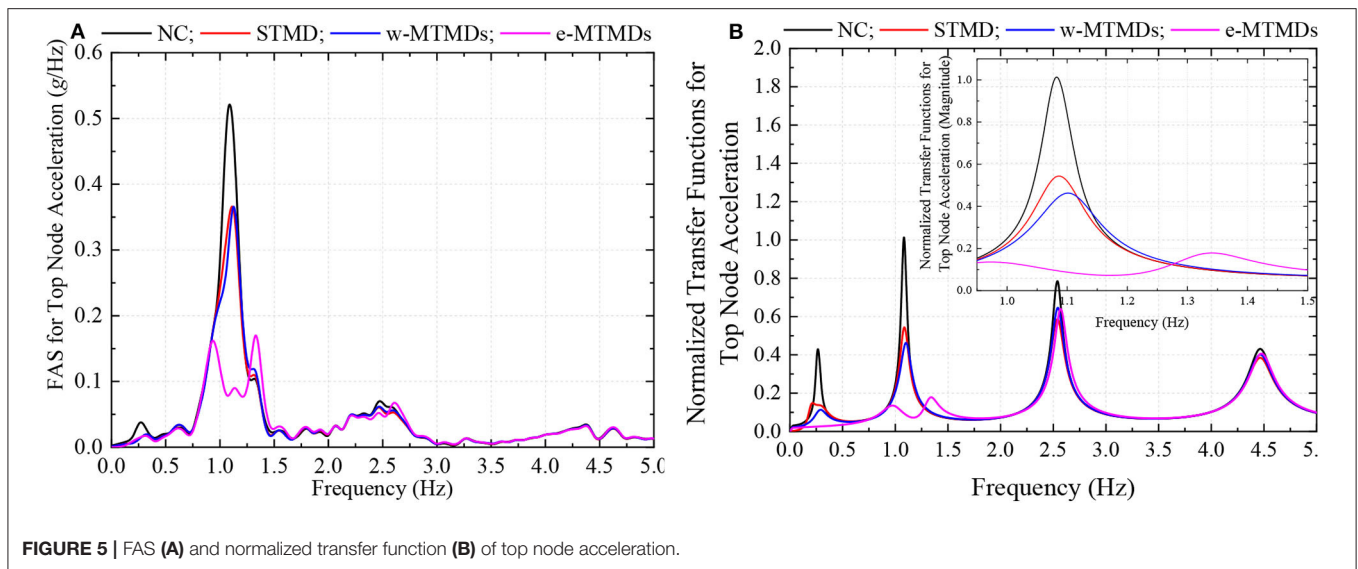


FIGURE 4 | FAS (A) and normalized transfer function (B) of top node displacement.



band around resonance with the first mode, the effectiveness is higher. In this band, there is no significant difference between the STMD and MTMDs. When the second mode period is close to the pulse period, the TMDs reduce the displacement by up to 40%, and the e-TMDs outperform the other schemes. It is noted that the structure experiences the highest displacement demand, and in such cases, response reduction is in the range of 15–30%. In general, the e-MTMD seems to be the most effective scheme. Similar observations can be made in **Figure 8B** which shows response reduction in acceleration. In this case, the e-MTMDs are clearly superior to the other schemes.

These results highlight that some degree of response reduction can be achieved in tall chimneys subjected to near-fault pulse-like ground motions by passive control schemes using TMDs. The effectiveness of the control schemes, however, differs a lot of ground motion characteristics, most importantly on its frequency content relative to the vibration frequencies of the structure. Since a structure, during its useful life, might face

ground motions with different frequency contents, such control schemes are not expected to be beneficial in all cases. However, the most critical cases which require control are those when the fundamental mode of the structure resonates with the ground motion pulse. In such cases, TMDs can be effective in controlling structural displacement demand. The TMDs presented in this research are optimized by maximizing their average performance against all the ground motions. The large variability in response reduction observed from the results presented here, a more focused optimization scheme might be more beneficial. For example, the TMDs can be optimized for a sub-set of ground motions which impose the largest demands on the uncontrolled structure. This and other advanced optimization schemes need further research, and might potentially provide better control, but it is clear that optimization schemes that are independent of ground motions, for example, that of Sadek et al. (1997) are not expected to be robust enough when the structure is subjected to near-fault ground motions.

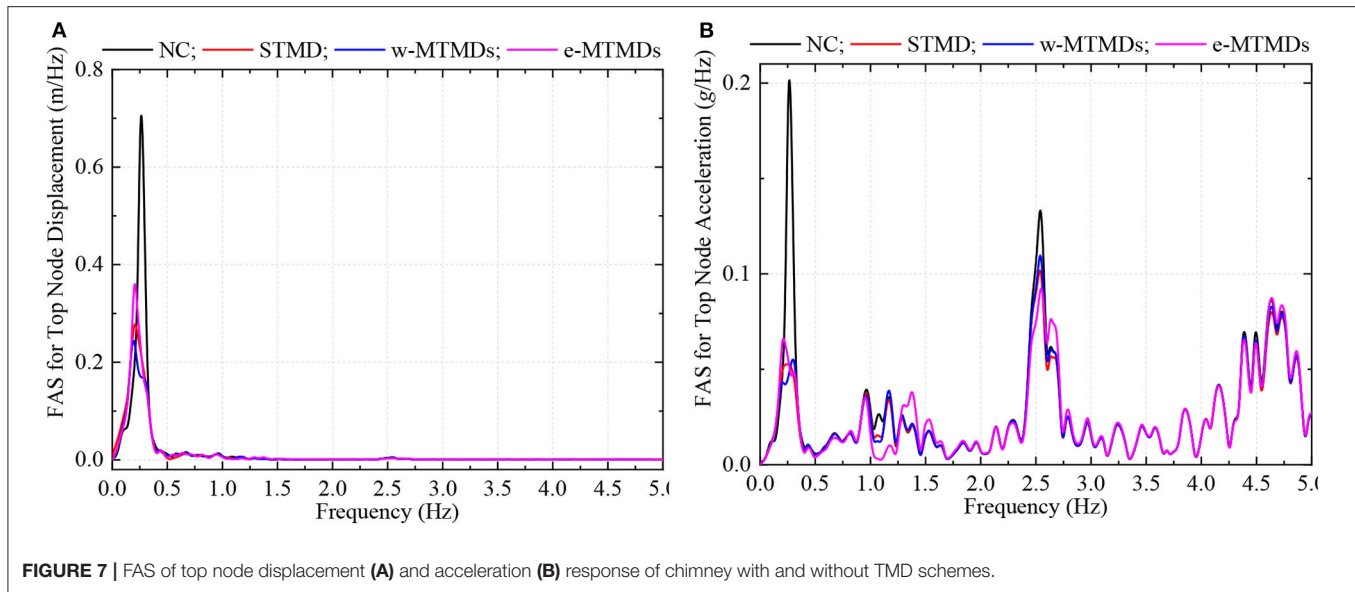


FIGURE 7 | FAS of top node displacement (A) and acceleration (B) response of chimney with and without TMD schemes.

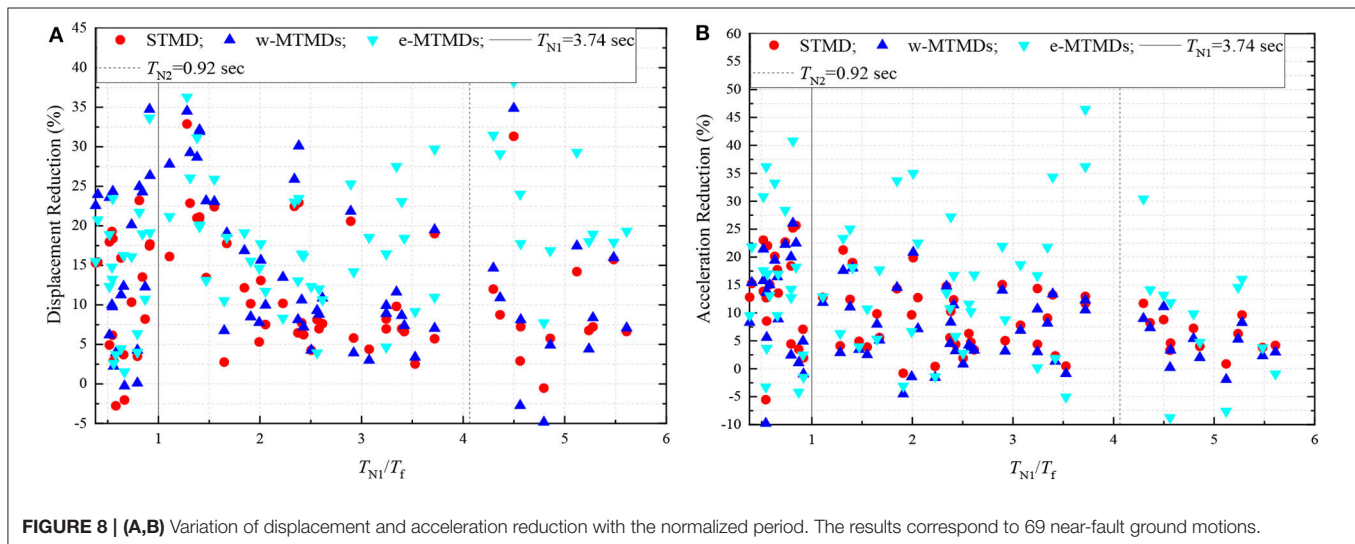


FIGURE 8 | (A,B) Variation of displacement and acceleration reduction with the normalized period. The results correspond to 69 near-fault ground motions.

CONCLUSIONS

The results discussed above lead to the following conclusions.

1. Passive TMDs optimized by maximizing average response reduction across a set of ground motions show large variability in control performance and offer limited effectiveness against most of the ground motions.
2. In the critical cases when the fundamental mode of the structure resonates with the ground motion pulse, structural displacements can be controlled by 10–35%.
3. Multiple TMDs, especially the e-MTMDs tuned to the first two modes of vibration provide more robust control than STMDs. This is more the case in acceleration control.
4. When the pulse period is much larger than the fundamental period of the structure, the TMDs don't provide effective control.
5. Traditional methods of optimization which are independent of ground motion do not provide effective and robust vibration control against near-fault ground motions. In this regard, the optimization needs to be target to a certain type of ground motions that are the most critical for the structure. This corresponds to situations when the first and the second vibration mode of the structure resonates with the pulse period. For such situations, e-MTMDs optimized for only those ground motions that resonate with the first and the second mode of the structure, might provide better control of displacement and acceleration demand. For

practical applications, this requires a good understanding of the impending hazard and the critical seismic sources near the construction site. The pulse period is dependent on earthquake magnitude (see Rupakhety et al., 2011) and can be estimated empirically. With such estimates, a more targeted optimization can be carried out so that the TMDs provide superior performance when subjected to the most critical ground motions.

DATA AVAILABILITY STATEMENT

The datasets generated for this study are available on request to the corresponding author.

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AUTHOR CONTRIBUTIONS

All authors listed have made a substantial, direct and intellectual contribution to the work, and approved it for publication.

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SUPPLEMENTARY MATERIAL

The Supplementary Material for this article can be found online at: <https://www.frontiersin.org/articles/10.3389/fbuil.2020.00092/full#supplementary-material>

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Conflict of Interest: The authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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